



## Effects of Partially Replacing Waste HDPE Plastics as Fine Aggregates on the Mechanical Properties of Self-Consolidating Concrete (SCC)

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**Abstract.** Artificial fine aggregate synthesized from waste HDPE plastic was incorporated into the concrete mix at 2.5%, 5%, 7.5%, 10%, and 12.5% by weight of the natural fine aggregate. Various tests were performed on both freshly mixed and hardened concrete. Concrete samples, including 150 mm cubes and 100x100x500 mm beams, were cast to evaluate mechanical properties such as compressive strength and flexural strength. The test results indicated that the mix with a 10% replacement of artificial fine aggregate (AFA) improved compressive strength by 6.8% and flexural strength by 4.9% compared to those using natural aggregates. This demonstrates that using plastic-based artificial aggregates enhances concrete properties and supports sustainability.

**Keywords:** Artificial Aggregate, Self compacting Concrete, Mechanical properties, HDPE plastic, Recycle plastic.

### 1Introduction

The literature review addresses the pressing issue of land filling, exacerbated by the increasing population and limited land availability. A significant part of this waste comprises plastic, notorious for its slow decomposition rate, taking approximately 500 years to break down. To mitigate this issue, recycling plastic instead of disposing of it in landfills is a promising solution. This approach is already applied in various sectors, including construction materials, fuel production from plastic waste, house hold goods, fabric, and clothing. The focus of this study is on the incorporation of plastic in the construction industry, particularly in concrete, to enhance sustainability and waste management.

Different types of plastics such as Polyethylene Terephthalate (PET), High-Density Polyethylene (HDPE), Polyvinyl Chloride (PVC), Low-Density Polyethylene

<sup>1</sup>Please note that the LNCSE Editorial assumes that all authors have used the western naming convention, with given names preceding surnames. This determines the structure of the

names in the running heads and the authorindex.

(LDPE), Polypropylene (PP), Polystyrene (PS), and Miscellaneous Plastics have been identified for potential use in construction. The study investigates the incorporation of these plastics into concrete and their effects on its properties.

<sup>1</sup> explored the impact of PET plastic waste on the compressive strength of concrete. They found that incorporating PET particles in varying dosages (0%, 5%, 10%, and 15%) resulted in reduced compressive strength, with a notable reduction at 15%. However, the 15% PET concrete showed higher resistance under sulfuric acid attack, suggesting a potential benefit in specific conditions.

<sup>2</sup> investigated the use of HDPE waste in concrete, with HDPE content ranging from 0-5% of the total mix weight. They observed a decrease in compressive strength beyond 0.5% HDPE addition, which could potentially be mitigated by treating the HDPE to enhance its bonding with the concrete matrix.

Further studies by <sup>3</sup>, Beno<sup>45</sup> examined shredded PET bottle waste as a partial substitute for natural aggregates in concrete. These studies consistently found that the incorporation of PET aggregates resulted in lower compressive, flexural, and tensile strengths compared to conventional concrete. Moreover, the strengths decreased with increasing PET content.

Research on replacing coarse aggregate with polyethylene (PE) in concrete showed a reduction in workability by 50 mm when substituting PE in the range of 5-15% with a water-to-cement ratio of 0.42. Similar trends were observed in studies utilizing various plastic waste materials, including shredded fibers from polythene bags, PET bottle fibers, granulated plastic waste, and PVC pipes.

A notable gap in the literature is the exploration of using plastic in fine aggregate form rather than as coarse aggregate. This aspect is crucial for sustainability, as it could reduce the reliance on naturally available aggregates. The present study aims to fill this gap by investigating the use of plastic as fine aggregate in self-compacting concrete (SCC) instead of conventional concrete. SCC offers enhanced flowability and compaction without the need for vibration, presenting an innovative approach to incorporating plastic waste in construction materials.

In conclusion, while significant research has been conducted on incorporating plastic waste into concrete, primarily focusing on coarse aggregate substitution, the potential for using plastic as fine aggregate remains underexplored. This study aims to address this gap, offering insights into the performance of SCC with plastic fine aggregates. Such an approach could provide substantial benefits in terms of sustainability and resource efficiency in the construction industry.

2. Experimental Work:  
2.1 Materials used:

In this study, Ordinary Portland cement of 43-grade was used, complying with IS(IndianStandards)code8112-1989.Table1providesadetailedaccountofthephysicalpropertiesofallmaterialsinvolved.Riversan dfromZoneII,whichmeetsthe requirements of IS: 383–1970 and has a specific gravity of 2.61, was selected asthenaturalfine aggregate (NFA).

High-Density Polyethylene (HDPE) plastic served as a replacement for the naturalfine aggregate. The HDPE used in the study exhibited tensile strength between 33 and40 MPa, flexural strength ranging from 15 to 55 MPa, and a flash point of 38°C. TheHDPE was processed into fine particles, and those that passed through 4.75mm sieveswereemployedasthe artificialfine aggregate (AFA).

For the coarse aggregate, crushed stone with a particle size of 20mm and a specificgravity of 2.76 was chosen. Given the aim to produce self-compacting concrete (SCC)instead of traditional concrete, a Polycarboxylate Ether (PCE)-based super plasticizerwas used to improve the mix's flow ability and workability while maintaining itsstructuralproperties<sup>6,7</sup>.

Table1Physicalpropertiesofmaterials

| Properties                       | Cement                | HDPE                             | FineAggregate<br>(FA) | CoarseAggreg<br>ate<br>(CA) |
|----------------------------------|-----------------------|----------------------------------|-----------------------|-----------------------------|
| Maximumparticle<br>size(mm)      | 5micron               | 4.75                             | 4.75                  | 10                          |
| Chemicalresistance               | chemicalresis<br>tant | Solvents,<br>alkalisAndaci<br>ds | -                     | -                           |
| Electricalresistivity<br>(ohm-m) | 1                     | 1016–1018                        |                       |                             |
| Finenessmodulus                  | 7                     | 3.12                             | 2.48                  | 8.72                        |
| Waterabsorption<br>(%)           | 13                    | 0.32                             | 2.68                  | 0.89                        |
| Density                          | 1.44g/cm <sup>3</sup> | 0.90–0.94<br>g/cm <sup>3</sup>   | 2630kg/m <sup>3</sup> | 2442kg/m <sup>3</sup>       |
| Specificgravity                  | 3.15                  | 0.864                            | 2.61                  | 2.76                        |

2.2.Casting and Mixing

Initially, experiments were conducted to attain M30 grade concrete, incorporating diverse quantities of fine aggregate (FA), coarse aggregate (CA), and superplasticizer. The aim of these trials was to develop self-compacting concrete (SCC) that demonstrates adequate flow, in accordance with EFNARC guidelines. These experiments were deemed necessary due to the absence of a standardized code offering a design mix for SCC.

Following numerous trials, a formulation was ultimately settled upon, meeting the criteria of ample flow and achieving strength of 30 MPa. This formulation is denoted as the control mix. The proportions for the control mix are delineated in Table 2, along with specifics for other formulations considered during the experimental phase. To investigate the influence of HDPE waste plastic as a substitute for fine aggregate (henceforth referred to as artificial aggregate) in concrete, various formulations were devised with a stepwise increase of 2.5% each, replacing natural fine aggregate. These substitutions spanned from 0% to 10%. The proportions for all formulations are elucidated in Table 2.

Table 2 Concrete Mix proportion

| Specimen<br>ode | CCement<br>g/) | (KAFA<br>% | AFA<br>(kg) | FA(Kg) | CA(Kg) | W/C  | Water(<br>kg) | Admixture(<br>Kg) |
|-----------------|----------------|------------|-------------|--------|--------|------|---------------|-------------------|
| Reference       | 500            | 0          | 0           | 880    | 880    | 0.50 | 225           | 8                 |
| HDPE-2.5        | 500            | 2.50       | 22          | 858    | 880    | 0.50 | 225           | 8                 |
| HDPE-5          | 500            | 5.00       | 44          | 836    | 880    | 0.50 | 225           | 8                 |
| HDPE-7.5        | 500            | 7.50       | 66          | 814    | 880    | 0.50 | 225           | 8                 |
| HDPE-10         | 500            | 10.00      | 88          | 792    | 880    | 0.50 | 225           | 8                 |
| HDPE-12.5       | 500            | 10.00      | 110         | 770    | 880    | 0.50 | 225           | 8                 |

These formulations underwent thorough examination for both fresh and hardened properties. Fresh properties were evaluated through slump flow and V box tests, with the results presented in Table 3. These tests offer valuable insights into the workability and flow characteristics of concrete formulations containing differing percentages of HDPE waste plastic as a substitute for natural fine aggregate.

Table3 Properties of fresh SCC prepared concrete.

| SpecimenCode | Slumpflow    |         | V-funneltimeinSec |       |
|--------------|--------------|---------|-------------------|-------|
|              | Experimental | Range   | Experimental      | Range |
| Reference    | 578          | 550-600 | 7.48              | <8    |
| HDPE-2.5     | 582          | 550-600 | 7.36              | <8    |
| HDPE-5       | 586          | 550-600 | 7.31              | <8    |
| HDPE-7.5     | 594          | 550-600 | 7.26              | <8    |
| HDPE-10      | 604          | 550-600 | 7.37              | <8    |
| HDPE-12.5    | 604          | 550-600 | 7.31              | <8    |

According to the fresh properties results outlined in Table 3, it can be deduced that all formulations fall within the specified range by the EFNARC code, classifying them as self-compacting concrete (SCC) with favourable flow characteristics. Noteworthy is that with the increase in the dosage of artificial fine aggregate (AFA) replacing natural fine aggregate (NFA), the slump flow also increased, reaching its peak diameter of 604mm for the formulation with 12.5% replacement. Furthermore, in the V-funnel test, it was noted that the formulation with 12.5% replacement required only 7.31 seconds emptying the funnel. These observations suggest an enhanced flow capacity and workability of concrete formulations with higher proportions of HDPE waste plastic substituting natural fine aggregate.

3.

3.1

Results and Discussions

Compressive Strength Results:

To examine the influence of artificial aggregate on the compressive strength or ultimate load-bearing capacity of the produced concrete, three cubes for each formulation underwent testing using a compressive testing machine (CTM) until failure. The outcomes of these tests are documented in Table 3 and visualized in Figures 1 and 2. These results offer valuable insights into the impact of replacing natural fine aggregate with HDPE waste plastic on the structural performance and load-bearing capacity of the concrete.

| SpecimenCode | Table4Compressivestrengthfor differentages |       |        |                                 |
|--------------|--------------------------------------------|-------|--------|---------------------------------|
|              | CompressiveStrength(Mpa)                   |       |        | Increase instrengthof 28days(%) |
|              |                                            |       |        |                                 |
|              | 3Days                                      | 7Days | 28Days |                                 |
| Reference    | 15.7                                       | 24.7  | 39.5   | -                               |
| HDPE-2.5     | 15.2                                       | 23.1  | 38.7   | -2.02                           |
| HDPE-5       | 15.9                                       | 26.05 | 40.3   | 2.02                            |
| HDPE-7.5     | 16.7                                       | 27.3  | 42.9   | 8.6                             |
| HDPE-10      | 17.25                                      | 29.2  | 43.5   | 10.12                           |
| HDPE-12.5    | 16.8                                       | 28.9  | 42.3   | 7.08                            |

The specimens underwent testing at 3 days, 7 days, and 28 days to observe the impact of curing on the strength when fine aggregate (FA) is replaced in concrete. The findings reveal that with increasing curing time, the compressive strength enhances irrespective of the dosage of FA replacement. The table also delineates the percentage increase in compressive strength values for 28 days in comparison to the reference mix.

Initial observations indicate a decrease in compressive strength by 2.02% for a 2.5% dosage. However, subsequently, the strength increases by 2.02% and 8.6% for HDPE 5 and HDPE 7.5 specimens and strength increase by 10.12% for HDPE 10 specimen. As the dosage increases after 10 HDPE, the strength tends to decline, which is 7.08% at HDPE 12.5 specimen, which is less than that of HDPE 10.

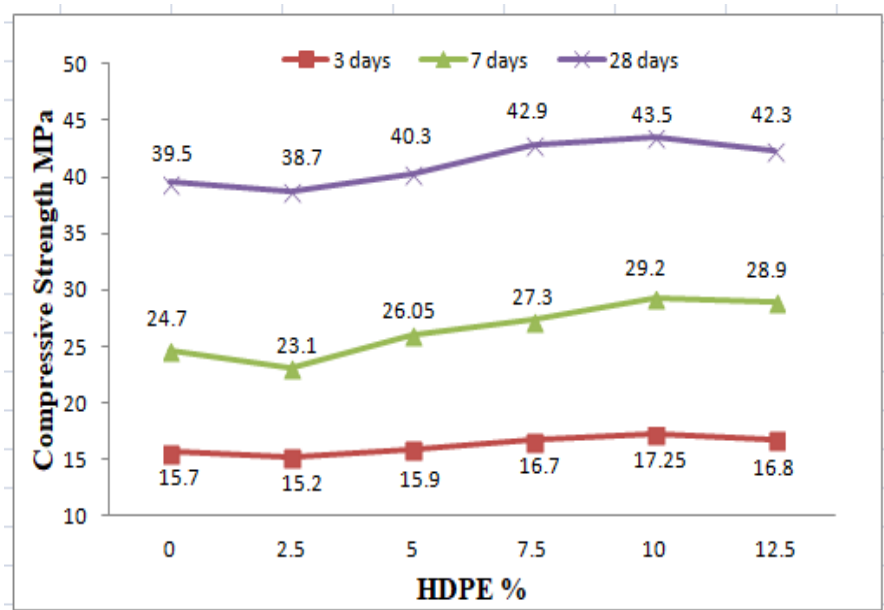


Fig.1PlotCompressivestrengthfordifferentagesanddosagesofAFA

3.2 Flexural Strength Results:

To evaluate the influence of artificial aggregate on the flexural strength capacity of the produced concrete, three beams measuring 75 x 10 x 10 mm were tested under three-point loading. Each mix was represented by a set of three beams, which were then subjected to testing under a compressive testing machine (CTM) until failure. The peak load values were recorded, and the flexural strength was determined using equation 1 based on these peak loads. The calculated results are summarized in Table 5 and depicted in Figures 3 and 4. These findings offer insights into the impact of replacing natural fine aggregate with HDPE waste plastic on the flexural strength of the concrete.

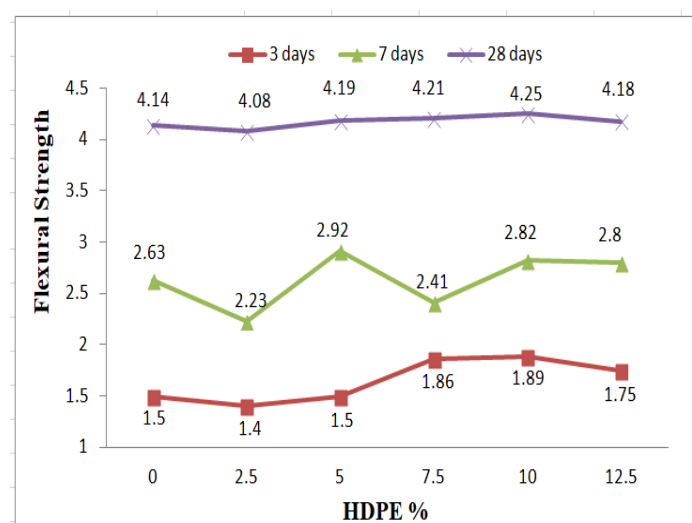
Table 5 Flexural strength for different ages.

| Flexural Strength (MPa) | Increase in    |
|-------------------------|----------------|
| Specimen Code           | strength of 28 |



|           | 3Days | 7Days | 28Days | days(%) |
|-----------|-------|-------|--------|---------|
| Reference | 1.5   | 2.63  | 4.14   | 0       |
| HDPE-2.5  | 1.4   | 2.23  | 4.08   | -1.2    |
| HDPE-5    | 1.5   | 2.92  | 4.19   | 1.2     |
| HDPE-7.5  | 1.86  | 2.41  | 4.21   | 1.69    |
| HDPE-10   | 1.89  | 2.82  | 4.25   | 2.65    |
| HDPE-12   | 1.75  | 2.80  | 4.18   | 0.96    |

Similar to the compressive strength testing, the beam sets were tested at 3, 7, and 28 days to evaluate their tensile strength. As expected, replacing natural fine aggregate (NFA) with artificial fine aggregate (AFA) did not significantly enhance tensile strength. However, mirroring the patterns seen in compressive strength, an increase in flexural strength was observed. The increases in strength for the 5%, 7.5%, 10% and 12.5% replacements were 1.2%, 1.69%, 2.65 and 0.96%, respectively. The optimal dosage, once again, seems to be 10%, as it yielded the highest increase in strength. These findings provide important insights into how substituting natural fine aggregate with HDPE waste plastic affects both the tensile and flexural strength of the concrete.



**Fig.3** Plot Flexural strength for different ages and dosages of AFA

#### 4. Conclusions

Based on the findings from this study, the following conclusions are drawn:

1. The fresh properties of self-compacting concrete improved as the amount of artificial fine aggregate (AFA) increased.
2. The slump flow test showed an increase in flow with higher AFA dosages, reaching a diameter of 604 mm, while the V-box test time decreased to 7.31 seconds at a 12.5% replacement.

3. Compressive strength initially dropped by 2.02% with a 2.5% replacement of natural fine aggregate (NFA) by AFA, then increased to 10.12% at a 10% replacement before declining again. The optimal replacement level is 10%.
4. Flexural strength initially decreased but achieved a maximum increase of 2.65% with a 10% replacement.
5. In conclusion, sustainable concrete can be produced by substituting natural fine aggregate with waste plastic, leading to environmental benefits and improved fresh and hardened properties of the concrete.

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